



Shear Stress in I Section Formulas

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List of 33 Shear Stress in I Section Formulas

Shear Stress in I Section 🗗

Shear Stress Distribution in Flange

1) Area of Flange or Area above Considered Section

$$\mathbf{A}_{\mathrm{abv}} = \mathrm{B} \cdot \left(rac{\mathrm{D}}{2} - \mathrm{y}
ight)$$

2) Distance of CG of Considered Area of Flange from Neutral Axis in I Section

$$ar{\mathbf{y}} = rac{1}{2} \cdot \left(rac{\mathrm{D}}{2} + \mathrm{y}
ight)^{2}$$

$$2252.5 \text{mm} = \frac{1}{2} \cdot \left(\frac{9000 \text{mm}}{2} + 5 \text{mm} \right)$$

3) Distance of Considered Section from Neutral Axis given Shear Stress in Flange

$$\mathbf{x} = \sqrt{rac{\mathrm{D}^2}{2} - rac{2 \cdot \mathrm{I}}{\mathrm{F_s}}} \cdot au_{\mathrm{beam}}$$

4) Distance of Lower Edge of Flange from Neutral Axis

$$y = \frac{d}{2}$$

$$\boxed{ 225 \text{mm} = \frac{450 \text{mm}}{2} }$$

5) Distance of Upper Edge of Flange from Neutral Axis

$$y = \frac{D}{2}$$



6) Inner Depth of I-section given Shear Stress in Lower Edge of Flange

$$d = \sqrt{D^2 - rac{8 \cdot I}{F_s} \cdot au_{beam}}$$

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$$8012.49 \text{mm} = \sqrt{(9000 \text{mm})^2 - \frac{8 \cdot 0.00168 \text{m}^4}{4.8 \text{kN}} \cdot 6 \text{MPa} }$$

7) Moment of Inertia of I section given Shear Stress in Lower Edge of Flange

$$\boxed{\mathbf{f}} \mathbf{I} = \frac{F_s}{8 \cdot \tau_{\mathrm{beam}}} \cdot \left(D^2 - d^2\right)$$

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$$\boxed{ \text{ex} \left[0.00808 \text{m}^4 = \frac{4.8 \text{kN}}{8 \cdot 6 \text{MPa}} \cdot \left(\left(9000 \text{mm} \right)^2 - \left(450 \text{mm} \right)^2 \right) \right] }$$

8) Moment of Inertia of Section for I-section

$$I = rac{F_{
m s}}{2 \cdot au_{
m beam}} \cdot \left(rac{D^2}{2} - y^2
ight)$$

Open Calculator

ex
$$0.0162\text{m}^4 = \frac{4.8\text{kN}}{2 \cdot 6\text{MPa}} \cdot \left(\frac{(9000\text{mm})^2}{2} - (5\text{mm})^2\right)$$

9) Outer Depth of I section given Shear Stress in Lower Edge of Flange

$$D = \sqrt{rac{8 \cdot I}{F_s} \cdot au_{beam} + d^2}$$

Open Calculator

10) Outer Depth of I-section given Shear Stress in Flange

$$D=4\cdot\sqrt{rac{2\cdot I}{F_s}\cdot au_{beam}+y^2}$$

$$\boxed{\texttt{ex}} 8197.585 \text{mm} = 4 \cdot \sqrt{\frac{2 \cdot 0.00168 \text{m}^4}{4.8 \text{kN}} \cdot 6 \text{MPa} + (5 \text{mm})^2}$$



11) Shear Force in Flange of I-section

$$ext{F}_{
m s} = rac{2 \cdot ext{I} \cdot au_{
m beam}}{rac{ ext{D}^2}{2} - ext{y}^2}$$

Open Calculator

12) Shear Force in Lower Edge of Flange in I-section

$$\mathbf{F}_{\mathrm{s}} = rac{8 \cdot I \cdot au_{\mathrm{beam}}}{D^2 - d^2}$$

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$$\boxed{\textbf{ex}} 0.998051 \text{kN} = \frac{8 \cdot 0.00168 \text{m}^4 \cdot 6 \text{MPa}}{\left(9000 \text{mm}\right)^2 - \left(450 \text{mm}\right)^2}$$

13) Shear Stress in Flange of I-section

$$au_{
m beam} = rac{F_{
m s}}{2 \cdot I} \cdot \left(rac{D^2}{2} - {
m y}^2
ight)$$

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14) Shear Stress in Lower Edge of Flange of I-section

$$au_{
m beam} = rac{{
m F}_{
m s}}{8 \cdot {
m I}} \cdot \left({
m D}^2 - {
m d}^2
ight)$$

Open Calculator

15) Width of Section given Area above Considered Section of Flange

$$B=rac{A_{abv}}{rac{D}{2}-y}$$



Shear Stress Distribution in Web

16) Distance of Considered Level from Neutral Axis at Junction of Top of Web

$$y = \frac{d}{2}$$

Open Calculator 🗗

$$225 \text{mm} = \frac{450 \text{mm}}{2}$$

17) Maximum Shear Force in I Section

$$F_{
m s}=rac{ au_{
m max}\cdot I\cdot b}{rac{B\cdot (D^2-d^2)}{8}+rac{b\cdot d^2}{8}}$$

Open Calculator

$$\boxed{ \text{ex} \ 0.128061 kN = \frac{11 MPa \cdot 0.00168 m^4 \cdot 7 mm}{\frac{100 mm \cdot \left((9000 mm)^2 - (450 mm)^2 \right)}{8} + \frac{7 mm \cdot (450 mm)^2}{8} } }$$

18) Maximum Shear Stress in I Section

$$\boxed{\textbf{fx}} \tau_{max} = \frac{F_s}{I \cdot b} \cdot \left(\frac{B \cdot \left(D^2 - d^2\right)}{8} + \frac{b \cdot d^2}{8} \right)$$

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$$\boxed{ \text{ex} \left[412.3045 \text{MPa} = \frac{4.8 \text{kN}}{0.00168 \text{m}^4 \cdot 7 \text{mm}} \cdot \left(\frac{100 \text{mm} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right)}{8} + \frac{7 \text{mm} \cdot (450 \text{mm})^2}{8} \right) \right] } \right] }$$

19) Moment of Flange Area about Neutral Axis

$$I = \frac{B \cdot (D^2 - d^2)}{8}$$

Open Calculator

$$\boxed{1.009969 \text{m}^4 = \frac{100 \text{mm} \cdot \left(\left(9000 \text{mm} \right)^2 - \left(450 \text{mm} \right)^2 \right)}{8}}$$

20) Moment of Inertia of I-Section given Maximum Shear Stress and Force

$$\mathbf{F}_{\mathrm{beam}} \cdot \mathbf{b} \cdot \left(rac{\mathbf{B} \cdot \left(\mathbf{D}^2 - \mathbf{d}^2
ight)}{8} + rac{\mathbf{b} \cdot \mathbf{d}^2}{8}
ight)$$

$$\underbrace{ \text{0.115445m}^4 = \frac{4.8 \text{kN}}{6 \text{MPa} \cdot 7 \text{mm}} \cdot \left(\frac{100 \text{mm} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right)}{8} + \frac{7 \text{mm} \cdot (450 \text{mm})^2}{8} \right) }$$





21) Moment of Inertia of I-Section given Shear Stress of Web 🗗

 $\mathbf{F} = rac{F_{
m s}}{ au_{
m beam} \cdot {
m b}} \cdot \left(rac{{
m B}}{8} \cdot \left({
m D}^2 - {
m d}^2
ight) + rac{{
m b}}{2} \cdot \left(rac{{
m d}^2}{4} - {
m y}^2
ight)
ight)$

Open Calculator 🗗

ex

$$\boxed{0.115445 \text{m}^4 = \frac{4.8 \text{kN}}{6 \text{MPa} \cdot 7 \text{mm}} \cdot \left(\frac{100 \text{mm}}{8} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right) + \frac{7 \text{mm}}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) \right)}$$

22) Moment of Inertia of Section given Shear Stress at Junction of Top of Web

 $I = rac{F_s \cdot B \cdot \left(D^2 - d^2
ight)}{8 \cdot au_{beam} \cdot b}$

Open Calculator

 $\underbrace{ \text{0.115425m}^4 = \frac{4.8 \text{kN} \cdot 100 \text{mm} \cdot \left(\left(9000 \text{mm} \right)^2 - \left(450 \text{mm} \right)^2 \right)}{8 \cdot 6 \text{MPa} \cdot 7 \text{mm}} }$

23) Moment of Shaded Area of Web about Neutral Axis

 $I = rac{\mathrm{b}}{2} \cdot \left(rac{\mathrm{d}^2}{4} - \mathrm{y}^2
ight)$

Open Calculator 🗗

 $\boxed{ 0.000177 \text{m}^4 = \frac{7 \text{mm}}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) }$

24) Shear Force at Junction of Top of Web

 $\mathbf{F}_{\mathrm{s}} = rac{8 \cdot \mathbf{I} \cdot \mathbf{b} \cdot au_{\mathrm{beam}}}{\mathbf{B} \cdot \left(\mathbf{D}^2 - \mathbf{d}^2
ight)}$

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$$= \frac{8 \cdot 0.00168 \text{m}^4 \cdot 7 \text{mm} \cdot 6 \text{MPa}}{100 \text{mm} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right)}$$

25) Shear Force in Web

$$\mathbf{F}_{\mathrm{s}} = rac{\mathrm{I}\cdot\mathrm{b}\cdot au_{\mathrm{beam}}}{rac{\mathrm{B}\cdot\left(\mathrm{D}^{2}-\mathrm{d}^{2}
ight)}{8}+rac{\mathrm{b}}{2}\cdot\left(rac{\mathrm{d}^{2}}{4}-\mathrm{y}^{2}
ight)}$$

$$= \frac{0.00168 m^4 \cdot 7 mm \cdot 6 MPa}{\frac{100 mm \cdot \left((9000 mm)^2 - (450 mm)^2 \right)}{8} + \frac{7 mm}{2} \cdot \left(\frac{(450 mm)^2}{4} - (5 mm)^2 \right)}$$





26) Shear Stress at Junction of Top of Web 🚰

 $au_{
m beam} = rac{F_{
m s} \cdot B \cdot \left(D^2 - d^2
ight)}{8 \cdot I \cdot b}$

Open Calculator

27) Shear Stress in Web

 $\boxed{ \tau_{beam} = \frac{F_s}{I \cdot b} \cdot \left(\frac{B}{8} \cdot \left(D^2 - d^2 \right) + \frac{b}{2} \cdot \left(\frac{d^2}{4} - y^2 \right) \right) }$

Open Calculator 🗗

ex

$$\boxed{412.3044 \text{MPa} = \frac{4.8 \text{kN}}{0.00168 \text{m}^4 \cdot 7 \text{mm}} \cdot \left(\frac{100 \text{mm}}{8} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right) + \frac{7 \text{mm}}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2 \right) + \frac{1}{2} \cdot \left(\frac{(450 \text{mm})^2}{4} - (5 \text{mm})^2$$

28) Thickness of Web

 $b = \frac{2 \cdot I}{\frac{d^2}{4} - y^2}$

Open Calculator

29) Thickness of Web given Maximum Shear Stress and Force 🖸

 $b = rac{B \cdot F_s \cdot \left(D^2 - d^2
ight)}{8 \cdot I \cdot au_{
m beam} - F_s \cdot d^2}$

Open Calculator 🗗

$$= \frac{100 \text{mm} \cdot 4.8 \text{kN} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right)}{8 \cdot 0.00168 \text{m}^4 \cdot 6 \text{MPa} - 4.8 \text{kN} \cdot (450 \text{mm})^2}$$

30) Thickness of Web given Shear Stress at Junction of Top of Web 🚰

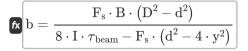
 $oldsymbol{f_s} b = rac{F_s \cdot B \cdot \left(D^2 - d^2
ight)}{8 \cdot I \cdot au_{beam}}$

$$= \frac{4.8 \text{kN} \cdot 100 \text{mm} \cdot \left((9000 \text{mm})^2 - (450 \text{mm})^2 \right)}{8 \cdot 0.00168 \text{m}^4 \cdot 6 \text{MPa} }$$





31) Thickness of Web given Shear Stress of Web



Open Calculator

32) Width of Section given Moment of Flange Area about Neutral Axis

$$\mathbf{E} = \frac{8 \cdot \mathbf{I}}{\mathbf{D}^2 - \mathbf{d}^2}$$

Open Calculator

$$\boxed{\textbf{ex} 0.166342 \text{mm} = \frac{8 \cdot 0.00168 \text{m}^4}{\left(9000 \text{mm}\right)^2 - \left(450 \text{mm}\right)^2}}$$

33) Width of Section given Shear Stress at Junction of Top of Web

$$\mathbf{F} = rac{ au_{
m beam} \cdot 8 \cdot I \cdot b}{ ext{F}_{
m s} \cdot \left(ext{D}^2 - ext{d}^2
ight)}$$

$$\boxed{ 1.455491 mm = \frac{6 M Pa \cdot 8 \cdot 0.00168 m^4 \cdot 7 mm}{4.8 k N \cdot \left(\left(9000 mm \right)^2 - \left(450 mm \right)^2 \right)} }$$



Variables Used

- A_{aby} Area of Section above Considered Level (Square Millimeter)
- **b** Thickness of Beam Web (Millimeter)
- B Width of Beam Section (Millimeter)
- **d** Inner Depth of I Section (Millimeter)
- **D** Outer Depth of I section (Millimeter)
- F_s Shear Force on Beam (Kilonewton)
- I Moment of Inertia of Area of Section (Meter4)
- **y** Distance from Neutral Axis (Millimeter)
- $\bar{\mathbf{y}}$ Distance of CG of Area from NA (Millimeter)
- τ_{beam} Shear Stress in Beam (Megapascal)
- $au_{ extbf{max}}$ Maximum Shear Stress on Beam (Megapascal)





Constants, Functions, Measurements used

- Function: sqrt, sqrt(Number) Square root function
- Measurement: Length in Millimeter (mm)
 Length Unit Conversion
- Measurement: Area in Square Millimeter (mm²)

 Area Unit Conversion
- Measurement: Pressure in Megapascal (MPa)
 Pressure Unit Conversion
- Measurement: Force in Kilonewton (kN)
 Force Unit Conversion
- Measurement: Second Moment of Area in Meter⁴ (m⁴)
 Second Moment of Area Unit Conversion





Check other formula lists

- Shear Stress in Circular Section Formulas
- Shear Stress in Rectangular Section Formulas
- Shear Stress in I Section Formulas 🗗

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