## Crack Width and Deflection of Prestress Concrete Members Formulas

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## List of 40 Crack Width and Deflection of Prestress Concrete Members Formulas

## Crack Width and Deflection of Prestress Concrete Members $\mathbb{B}$

## Calculation of Crack Width

1) Average Strain at Selected Level given Crack Width
$\mathrm{fx} \varepsilon_{\mathrm{m}}=\frac{\mathrm{W}_{\mathrm{cr}} \cdot\left(1+\left(2 \cdot \frac{\mathrm{acr}-\mathrm{C}_{\text {min }}}{\mathrm{h}-\mathrm{x}}\right)\right)}{3 \cdot \mathrm{acr}}$
ex $0.0005=\frac{0.49 \mathrm{~mm} \cdot\left(1+\left(2 \cdot \frac{2.51 \mathrm{~cm}-9.48 \mathrm{~cm}}{20.1 \mathrm{~cm}-50 \mathrm{~mm}}\right)\right)}{3 \cdot 2.51 \mathrm{~cm}}$
2) Center to Center Spacing given Shortest Distance
$f \mathbf{x}=2 \cdot \sqrt{\left(\operatorname{acr}+\left(\frac{D}{2}\right)\right)^{2}-\left(d^{\prime 2}\right)}$
$\operatorname{ex} 54.10324 \mathrm{~cm}=2 \cdot \sqrt{\left(2.51 \mathrm{~cm}+\left(\frac{0.5 \mathrm{~m}}{2}\right)\right)^{2}-\left((50.01 \mathrm{~mm})^{2}\right)}$
3) Crack Width on Surface of Section
$\mathrm{fx} \mathrm{W}_{\text {cr }}=\frac{3 \cdot \mathrm{acr} \cdot \varepsilon_{\mathrm{m}}}{1+\left(2 \cdot \frac{\mathrm{acr}-\mathrm{C}_{\text {min }}}{\mathrm{h}-\mathrm{x}}\right)}$
ex $0.490099 \mathrm{~mm}=\frac{3 \cdot 2.51 \mathrm{~cm} \cdot 0.0005}{1+\left(2 \cdot \frac{2.51 \mathrm{~cm}-9.48 \mathrm{~cm}}{20.1 \mathrm{~cm}-50 \mathrm{~mm}}\right)}$
4) Depth of Neutral Axis given Crack Width
$\mathrm{fx} \mathrm{x}=\mathrm{h}-\left(2 \cdot \frac{\mathrm{acr}-\mathrm{C}_{\text {min }}}{3 \cdot \operatorname{acr} \cdot \varepsilon}-1\right)$
ex $3052.077 \mathrm{~mm}=20.1 \mathrm{~cm}-\left(2 \cdot \frac{2.51 \mathrm{~cm}-9.48 \mathrm{~cm}}{3 \cdot 2.51 \mathrm{~cm} \cdot 1.0001}-1\right)$
5) Diameter of Longitudinal Bar given Shortest Distance
$\mathrm{fx} D=\left(\sqrt{\left(\frac{\mathrm{z}}{2}\right)^{2}+\mathrm{d}^{\prime 2}}-\mathrm{acr}\right) \cdot 2$
$\operatorname{ex} 0.04982 \mathrm{~m}=\left(\sqrt{\left(\frac{40 \mathrm{~A}}{2}\right)^{2}+(50.01 \mathrm{~mm})^{2}}-2.51 \mathrm{~cm}\right) \cdot 2$
6) Effective Cover given Shortest Distance
$f \mathbf{x} d^{\prime}=\sqrt{\left(\operatorname{acr}+\left(\frac{D}{2}\right)\right)^{2}-\left(\frac{\mathrm{z}}{2}\right)^{2}}$
$e x 275.1 \mathrm{~mm}=\sqrt{\left(2.51 \mathrm{~cm}+\left(\frac{0.5 \mathrm{~m}}{2}\right)\right)^{2}-\left(\frac{40 \mathrm{~A}}{2}\right)^{2}}$
7) Minimum Clear Cover given Crack Width
$f \times \mathrm{C}_{\min }=\operatorname{acr}-\frac{\left(\left(\frac{3 \cdot \text { acr } \cdot \varepsilon_{\mathrm{m}}}{\mathrm{W}_{\mathrm{cr}}}\right)-1\right) \cdot(\mathrm{h}-\mathrm{x})}{2}$
ex $9.479883 \mathrm{~cm}=2.51 \mathrm{~cm}-\frac{\left(\left(\frac{3 \cdot 2.51 \mathrm{~cm} \cdot 0.0005}{0.49 \mathrm{~mm}}\right)-1\right) \cdot(20.1 \mathrm{~cm}-50 \mathrm{~mm})}{2}$

## Evaluation of Average Strain and Neutral Axis Depth

8) Area of Prestressing Steel given Tension Force
$\mathrm{fx} \mathrm{As}=\frac{\mathrm{N}_{\mathrm{u}}}{\mathrm{E}_{\mathrm{p}} \cdot \varepsilon}$
ex $26.31316 \mathrm{~mm}^{2}=\frac{1000 \mathrm{~N}}{38 \mathrm{~kg} / \mathrm{cm}^{3} \cdot 1.0001}$
9) Average Strain under Tension
$\mathrm{fx} \varepsilon_{\mathrm{m}}=\varepsilon_{1}-\frac{\mathrm{W}_{\mathrm{cr}} \cdot(\mathrm{h}-\mathrm{x}) \cdot\left(\mathrm{D}_{\mathrm{CC}}-\mathrm{x}\right)}{3 \cdot \mathrm{E}_{\mathrm{s}} \cdot \mathrm{A}_{\mathrm{s}} \cdot\left(\mathrm{L}_{\mathrm{eff}}-\mathrm{x}\right)}$
ex $0.000514=0.000514-\frac{0.49 \mathrm{~mm} \cdot(12.01 \mathrm{~m}-50 \mathrm{~mm}) \cdot(4.5 \mathrm{~m}-50 \mathrm{~mm})}{3 \cdot 200000 \mathrm{MPa} \cdot 500 \mathrm{~mm}^{2} \cdot(50.25 \mathrm{~m}-50 \mathrm{~mm})}$
10) Compression Force for Prestressed Section
$\mathrm{fx}_{\mathrm{x}} \mathrm{C}_{\mathrm{c}}=\mathrm{As} \cdot \mathrm{E}_{\mathrm{p}} \cdot \varepsilon$
ex $767.6768 \mathrm{~N}=20.2 \mathrm{~mm}^{2} \cdot 38 \mathrm{~kg} / \mathrm{cm}^{3} \cdot 1.0001$
11) Couple Force of Cross Section
$f \mathrm{f}=0.5 \cdot \mathrm{E}_{\mathrm{c}} \cdot \varepsilon_{\mathrm{c}} \cdot \mathrm{x} \cdot \mathrm{W}_{\mathrm{cr}}$
Open Calculator
ex $0.00325 \mathrm{kN}=0.5 \cdot 0.157 \mathrm{MPa} \cdot 1.69 \cdot 50 \mathrm{~mm} \cdot 0.49 \mathrm{~mm}$
12) Depth of Neutral Axis given Couple Force of Cross Section
$\mathrm{fx} \mathrm{x}=\frac{\mathrm{C}}{0.5 \cdot \mathrm{E}_{\mathrm{c}} \cdot \varepsilon_{\mathrm{c}} \cdot \mathrm{W}_{\mathrm{cr}}}$
ex $430.7305 \mathrm{~mm}=\frac{0.028 \mathrm{kN}}{0.5 \cdot 0.157 \mathrm{MPa} \cdot 1.69 \cdot 0.49 \mathrm{~mm}}$
13) Height of Crack Width at Soffit given Average Strain
$f \mathbf{x} h=\left(\frac{\left(\varepsilon_{1}-\varepsilon_{\mathrm{m}}\right) \cdot\left(3 \cdot \mathrm{E}_{\mathrm{s}} \cdot \mathrm{A}_{\mathrm{s}} \cdot(\mathrm{d}-\mathrm{x})\right)}{\mathrm{W}_{\mathrm{cr}} \cdot\left(\mathrm{D}_{\mathrm{CC}}-\mathrm{x}\right)}\right)+\mathrm{x}$
$67415.78 \mathrm{~m}=\left(\frac{(0.000514-0.0005) \cdot\left(3 \cdot 200000 \mathrm{MPa} \cdot 500 \mathrm{~mm}^{2} \cdot(85 \mathrm{~mm}-50 \mathrm{~mm})\right)}{0.49 \mathrm{~mm} \cdot(4.5 \mathrm{~m}-50 \mathrm{~mm})}\right)+50 \mathrm{~mm}$
14) Modulus of Elasticity of Concrete given Couple Force of Cross-Section
$f \mathrm{x} \mathrm{E}_{\mathrm{c}}=\frac{\mathrm{C}}{0.5 \cdot \varepsilon_{\mathrm{c}} \cdot \mathrm{x} \cdot \mathrm{W}_{\mathrm{cr}}}$
ex $1.352494 \mathrm{MPa}=\frac{0.028 \mathrm{kN}}{0.5 \cdot 1.69 \cdot 50 \mathrm{~mm} \cdot 0.49 \mathrm{~mm}}$
15) Modulus of Elasticity of Prestressed Steel given Compression Force
$f \times \mathrm{E}_{\mathrm{p}}=\frac{\mathrm{C}_{\mathrm{c}}}{\mathrm{As} \cdot \varepsilon}$
ex $37.125 \mathrm{~kg} / \mathrm{cm}^{3}=\frac{750 \mathrm{~N}}{20.2 \mathrm{~mm}^{2} \cdot 1.0001}$
16) Strain at Selected Level given Average Strain under Tension
$f \mathbf{x} \varepsilon_{1}=\varepsilon_{\mathrm{m}}+\frac{\mathrm{W}_{\mathrm{cr}} \cdot(\mathrm{h}-\mathrm{x}) \cdot\left(\mathrm{D}_{\mathrm{CC}}-\mathrm{x}\right)}{3 \cdot \mathrm{E}_{\mathrm{s}} \cdot \mathrm{A}_{\mathrm{s}} \cdot\left(\mathrm{L}_{\mathrm{eff}}-\mathrm{x}\right)}$
ex $0.0005=0.0005+\frac{0.49 \mathrm{~mm} \cdot(12.01 \mathrm{~m}-50 \mathrm{~mm}) \cdot(4.5 \mathrm{~m}-50 \mathrm{~mm})}{3 \cdot 200000 \mathrm{MPa} \cdot 500 \mathrm{~mm}^{2} \cdot(50.25 \mathrm{~m}-50 \mathrm{~mm})}$
17) Strain given Couple Force of Cross Section
$\mathrm{fx} \varepsilon_{\mathrm{c}}=\frac{\mathrm{C}}{0.5 \cdot \mathrm{E}_{\mathrm{c}} \cdot \mathrm{x} \cdot \mathrm{W}_{\mathrm{cr}}}$
Open Calculator ©
ex $14.55869=\frac{0.028 \mathrm{kN}}{0.5 \cdot 0.157 \mathrm{MPa} \cdot 50 \mathrm{~mm} \cdot 0.49 \mathrm{~mm}}$
18) Strain in Longitudinal Reinforcement given Tension Force
$f \mathrm{x} \varepsilon s=\frac{\mathrm{N}_{\mathrm{u}}}{\mathrm{A}_{\mathrm{s}} \cdot \mathrm{Es}}$
ex $10=\frac{1000 \mathrm{~N}}{500 \mathrm{~mm}^{2} \cdot 200000}$
19) Strain in Prestressed Steel given Tension Force ©
$\mathrm{fx} \varepsilon=\frac{\mathrm{N}_{\mathrm{u}}}{\mathrm{As} \cdot \mathrm{E}_{\mathrm{p}}}$
ex $1.302762=\frac{1000 \mathrm{~N}}{20.2 \mathrm{~mm}^{2} \cdot 38 \mathrm{~kg} / \mathrm{cm}^{3}}$
20) Width of Section given Couple Force of Cross Section
$f \mathrm{fx} \mathrm{W}_{\mathrm{cr}}=\frac{\mathrm{C}}{0.5 \cdot \mathrm{E}_{\mathrm{c}} \cdot \varepsilon \cdot \mathrm{x}}$
ex $7.133045 \mathrm{~mm}=\frac{0.028 \mathrm{kN}}{0.5 \cdot 0.157 \mathrm{MPa} \cdot 1.0001 \cdot 50 \mathrm{~mm}}$

## Deflection

21) Deflection due to Self Weight given Short Term Deflection at Transfer
$\mathrm{fx} \Delta \mathrm{sw}=\Delta \mathrm{po}+\Delta \mathrm{st}$
ex $5 \mathrm{~cm}=2.5 \mathrm{~cm}+2.50 \mathrm{~cm}$
22) Short Term Deflection at Transfer
fx $\Delta \mathrm{st}=-\Delta \mathrm{po}+\Delta \mathrm{sw}$
$\mathrm{ex} 2.6 \mathrm{~cm}=-2.5 \mathrm{~cm}+5.1 \mathrm{~cm}$

## Deflection due to Prestressing Force

23) Deflection due to Prestressing for Parabolic Tendon
$\mathrm{fx} \delta=\left(\frac{5}{384}\right) \cdot\left(\frac{\mathrm{W}_{\mathrm{up}} \cdot \mathrm{L}^{4}}{\mathrm{E} \cdot \mathrm{I}_{\mathrm{A}}}\right)$
ex $48.08571 \mathrm{~m}=\left(\frac{5}{384}\right) \cdot\left(\frac{0.842 \mathrm{kN} / \mathrm{m} \cdot(5 \mathrm{~m})^{4}}{15 \mathrm{~Pa} \cdot 9.5 \mathrm{~m}^{4}}\right)$
24) Deflection due to Prestressing for Singly Harped Tendon
$f \mathrm{fx}=\frac{\mathrm{Ft} \cdot \mathrm{L}^{3}}{48 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{p}}}$
ex $48.08642 \mathrm{~m}=\frac{311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{48 \cdot 15 \mathrm{~Pa} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}$
25) Deflection due to Prestressing Force before Losses when Short Term Deflection at Transfer
$\mathrm{fx} \Delta \mathrm{po}=\Delta \mathrm{sw}-\Delta \mathrm{st}$
ex $2.6 \mathrm{~cm}=5.1 \mathrm{~cm}-2.50 \mathrm{~cm}$
26) Deflection due to Prestressing given Doubly Harped Tendon
$\mathrm{fx} \delta=\frac{\mathrm{a} \cdot\left(\mathrm{a}^{2}\right) \cdot \mathrm{Ft} \cdot \mathrm{L}^{3}}{24 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{p}}}$
$\mathbf{e x} 49.24049 \mathrm{~m}=\frac{0.8 \cdot\left((0.8)^{2}\right) \cdot 311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{24 \cdot 15 \mathrm{~Pa} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}$
27) Flexural Rigidity given Deflection due to Prestressing for Doubly Harped Tendon
$f \mathrm{fx}=\frac{\mathrm{a} \cdot\left(\mathrm{a}^{2}\right) \cdot \mathrm{Ft} \cdot \mathrm{L}^{3}}{24 \cdot \delta}$
Open Calculator
ex $17.27512 \mathrm{~N}^{*} \mathrm{~m}^{2}=\frac{0.8 \cdot\left((0.8)^{2}\right) \cdot 311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{24 \cdot 48.1 \mathrm{~m}}$
28) Flexural Rigidity given Deflection due to Prestressing for Parabolic Tendon
$f \times \mathrm{EI}=\left(\frac{5}{384}\right) \cdot\left(\frac{\mathrm{W}_{\mathrm{up}} \cdot \mathrm{L}^{4}}{\delta}\right)$
Open Calculator
ex $0.014246 \mathrm{~N}^{*} \mathrm{~m}^{2}=\left(\frac{5}{384}\right) \cdot\left(\frac{0.842 \mathrm{kN} / \mathrm{m} \cdot(5 \mathrm{~m})^{4}}{48.1 \mathrm{~m}}\right)$
29) Flexural Rigidity given Deflection due to Prestressing for Singly Harped Tendon
$f \mathrm{EX}=\frac{\mathrm{Ft} \cdot \mathrm{L}^{3}}{48 \cdot \delta}$
Open Calculator
ex $16.87024 \mathrm{~N}^{*} \mathrm{~m}^{2}=\frac{311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{48 \cdot 48.1 \mathrm{~m}}$
30) Length of Span given Deflection due to Prestressing for Doubly Harped Tendon
$f \times L=\left(\frac{\delta \cdot 48 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{p}}}{\mathrm{a} \cdot\left(4-3 \cdot \mathrm{a}^{2}\right) \cdot \mathrm{Ft}}\right)^{\frac{1}{3}}$
ex $4.219812 \mathrm{~m}=\left(\frac{48.1 \mathrm{~m} \cdot 48 \cdot 15 \mathrm{~Pa} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}{0.8 \cdot\left(4-3 \cdot(0.8)^{2}\right) \cdot 311.6 \mathrm{~N}}\right)^{\frac{1}{3}}$
31) Length of Span given Deflection due to Prestressing for Singly Harped Tendon
$f \mathbf{x}=\left(\frac{\delta \cdot 48 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{p}}}{\mathrm{Ft}}\right)^{\frac{1}{3}}$
ex $5.000471 \mathrm{~m}=\left(\frac{48.1 \mathrm{~m} \cdot 48 \cdot 15 \mathrm{~Pa} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}{311.6 \mathrm{~N}}\right)^{\frac{1}{3}}$
32) Moment of Inertia for Deflection due to Prestressing for Parabolic Tendon
$\mathrm{fx}_{\mathrm{x}} \mathrm{I}=\left(\frac{5}{384}\right) \cdot\left(\frac{\mathrm{W}_{\mathrm{up}} \cdot \mathrm{L}^{4}}{\mathrm{e}}\right)$
ex $137.0443 \mathrm{~kg} \cdot \mathrm{~m}^{2}=\left(\frac{5}{384}\right) \cdot\left(\frac{0.842 \mathrm{kN} / \mathrm{m} \cdot(5 \mathrm{~m})^{4}}{50 \mathrm{~Pa}}\right)$
33) Moment of Inertia for Deflection due to Prestressing in Doubly Harped Tendon
$f \mathrm{fx}=\frac{\mathrm{a} \cdot\left(\mathrm{a}^{2}\right) \cdot \mathrm{Ft} \cdot \mathrm{L}^{3}}{48 \cdot \mathrm{e} \cdot \delta}$
ex $0.172751 \mathrm{~kg} \cdot \mathrm{~m}^{2}=\frac{0.8 \cdot\left((0.8)^{2}\right) \cdot 311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{48 \cdot 50 \mathrm{~Pa} \cdot 48.1 \mathrm{~m}}$
34) Moment of Inertia for Deflection due to Prestressing of Singly Harped Tendon
$\mathrm{f} \mathrm{I}_{\mathrm{p}}=\frac{\mathrm{Ft} \cdot \mathrm{L}^{3}}{48 \cdot \mathrm{e} \cdot \delta}$
ex $0.337405 \mathrm{~kg} \cdot \mathrm{~m}^{2}=\frac{311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{48 \cdot 50 \mathrm{~Pa} \cdot 48.1 \mathrm{~m}}$
35) Uplift Thrust given Deflection due to Prestressing for Doubly Harped Tendon
$f \mathrm{Ft}=\frac{\delta \cdot 24 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{p}}}{\mathrm{a} \cdot\left(3-4 \cdot \mathrm{a}^{2}\right) \cdot \mathrm{L}^{3}}$
ex $442.7386 \mathrm{~N}=\frac{48.1 \mathrm{~m} \cdot 24 \cdot 15 \mathrm{~Pa} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}{0.8 \cdot\left(3-4 \cdot(0.8)^{2}\right) \cdot(5 \mathrm{~m})^{3}}$
36) Uplift Thrust given Deflection due to Prestressing for Singly Harped Tendon
$\mathrm{fx} \mathrm{Ft}=\frac{\delta \cdot 48 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{p}}}{\mathrm{L}^{3}}$
Open Calculator ك
ex $311.688 \mathrm{~N}=\frac{48.1 \mathrm{~m} \cdot 48 \cdot 15 \mathrm{~Pa} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}{(5 \mathrm{~m})^{3}}$
37) Uplift Thrust when Deflection due to Prestressing for Parabolic Tendon
$f \mathbf{x} \mathrm{~W}_{\mathrm{up}}=\frac{\delta \cdot 384 \cdot \mathrm{E} \cdot \mathrm{I}_{\mathrm{A}}}{5 \cdot \mathrm{~L}^{4}}$
ex $0.84225 \mathrm{kN} / \mathrm{m}=\frac{48.1 \mathrm{~m} \cdot 384 \cdot 15 \mathrm{~Pa} \cdot 9.5 \mathrm{~m}^{4}}{5 \cdot(5 \mathrm{~m})^{4}}$
38) Young's Modulus given Deflection due to Prestressing for Doubly Harped Tendon
$\mathrm{fx}_{\mathrm{x}} \mathrm{E}=\frac{\mathrm{a} \cdot\left(3-4 \cdot \mathrm{a}^{2}\right) \cdot \mathrm{Ft} \cdot \mathrm{L}^{3}}{48 \cdot \delta \cdot \mathrm{I}_{\mathrm{p}}}$
ex $5.278509 \mathrm{~Pa}=$

$$
\frac{0.8 \cdot\left(3-4 \cdot(0.8)^{2}\right) \cdot 311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{48 \cdot 48.1 \mathrm{~m} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}
$$

39) Young's Modulus given Deflection due to Prestressing for Parabolic Tendon
$\mathrm{fx}_{\mathrm{x}}^{\mathrm{E}}=\left(\frac{5}{384}\right) \cdot\left(\frac{\mathrm{W}_{\mathrm{up}} \cdot \mathrm{L}^{4}}{\delta \cdot \mathrm{I}_{\mathrm{A}}}\right)$
ex $14.99554 \mathrm{~Pa}=\left(\frac{5}{384}\right) \cdot\left(\frac{0.842 \mathrm{kN} / \mathrm{m} \cdot(5 \mathrm{~m})^{4}}{48.1 \mathrm{~m} \cdot 9.5 \mathrm{~m}^{4}}\right)$
40) Young's Modulus given Deflection due to Prestressing for Singly Harped Tendon
$\mathrm{fx} \mathrm{E}=\frac{\mathrm{Ft} \cdot \mathrm{L}^{3}}{48 \cdot \delta \cdot \mathrm{I}_{\mathrm{p}}}$
ex $14.99576 \mathrm{~Pa}=\frac{311.6 \mathrm{~N} \cdot(5 \mathrm{~m})^{3}}{48 \cdot 48.1 \mathrm{~m} \cdot 1.125 \mathrm{~kg} \cdot \mathrm{~m}^{2}}$

## Variables Used

- a Part of Span Length
- $\mathbf{A}_{\mathbf{s}}$ Area of Reinforcement (Square Millimeter)
- acr Shortest Distance (Centimeter)
- As Area of Prestressing Steel (Square Millimeter)
- C Couple Force (Kilonewton)
- $\mathbf{C}_{\mathbf{c}}$ Total Compression on Concrete (Newton)
- C min $_{\text {minimum }}$ Clear Cover (Centimeter)
- d Effective Depth of Reinforcement (Millimeter)
- d' Effective Cover (Millimeter)
- D Diameter of Longitudinal Bar (Meter)
- DCc Distance from Compression to Crack Width (Meter)
- e Elastic Modulus (Pascal)
- E Young's Modulus (Pascal)
- $\mathbf{E}_{\mathbf{c}}$ Modulus of Elasticity of Concrete (Megapascal)
- $\mathbf{E}_{\mathbf{p}}$ Prestressed Young's Modulus (Kilogram per Cubic Centimeter)
- $\mathbf{E}_{\mathbf{s}}$ Modulus of Elasticity of Steel Reinforcement (Megapascal)
- El Flexural Rigidity (Newton Square Meter)
- Es Modulus of Elasticity of Steel
- Ft Thrust Force (Newton)
- h Total Depth (Centimeter)
- h Height of Crack (Meter)
- $\mathbf{I}_{\mathrm{A}}$ Second Moment of Area (Meter ${ }^{4}$ )
- $\mathbf{I}_{\mathbf{p}}$ Moment of Inertia in Prestress (Kilogram Square Meter)
- L Span Length (Meter)
- Leff Effective Length (Meter)
- $\mathbf{N}_{\mathbf{u}}$ Tension Force (Newton)
- s Center to Center Spacing (Centimeter)
- W $\mathbf{\text { cr }}$ Crack Width (Millimeter)
- Wup Upward Thrust (Kilonewton per Meter)
- X Depth of Neutral Axis (Millimeter)
- z Center-to-center Distance (Angstrom)
- $\delta$ Deflection due to Moments on Arch Dam (Meter)
- $\Delta$ po Deflection due to Prestressing Force (Centimeter)
- $\Delta$ st Short Term Deflection (Centimeter)
- $\boldsymbol{\Delta} \mathbf{s w}$ Deflection due to Self Weight (Centimeter)
- $\varepsilon$ Strain
- $\varepsilon_{1}$ Strain at Selected Level
- $\varepsilon_{\mathbf{c}}$ Strain in Concrete
- $\varepsilon_{m}$ Average Strain
- $\boldsymbol{\varepsilon S}$ Strain in Longitudinal Reinforcement


## Constants, Functions, Measurements used

- Function: sqrt, sqrt(Number)

Square root function

- Measurement: Length in Millimeter (mm), Centimeter (cm), Meter (m), Angstrom (A) Length Unit Conversion
- Measurement: Area in Square Millimeter (mm²) Area Unit Conversion $\mathcal{E}$
- Measurement: Pressure in Megapascal (MPa), Pascal (Pa)

Pressure Unit Conversion

- Measurement: Force in Newton (N), Kilonewton (kN)

Force Unit Conversion

- Measurement: Surface Tension in Kilonewton per Meter (kN/m) Surface Tension Unit Conversion
- Measurement: Density in Kilogram per Cubic Centimeter (kg/cm ${ }^{3}$ ) Density Unit Conversion
- Measurement: Moment of Inertia in Kilogram Square Meter (kg•m²) Moment of Inertia Unit Conversion
- Measurement: Second Moment of Area in Meter ${ }^{4}\left(m^{4}\right)$

Second Moment of Area Unit Conversion

- Measurement: Flexural Rigidity in Newton Square Meter ( $\mathrm{N}^{*} \mathrm{~m}^{2}$ )

Flexural Rigidity Unit Conversion

## Check other formula lists

- Analysis of Prestressing and Bending Stresses Formulas
- Crack Width and Deflection of Prestress Concrete Members Formulas
- General Principles of Prestressed Concrete Formulas
- Transmission of Prestress Formulas

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