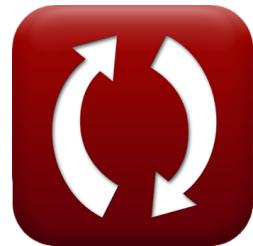


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Columns of Special Materials Formulas

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List of 21 Columns of Special Materials Formulas

Columns of Special Materials

Aluminium Column Design

1) Critical Slenderness Ratio for Aluminium Columns

fx
$$\lambda = \sqrt{\frac{51000000}{\frac{Q}{A}}}$$

[Open Calculator !\[\]\(de95854c7ee024cfadc48187bbb781b2_img.jpg\)](#)

ex
$$65.27367 = \sqrt{\frac{51000000}{\frac{633.213N}{52900mm^2}}}$$

2) Ultimate Load per Area for Aluminium Columns

fx
$$P = (34000 - 88 \cdot \lambda) \cdot A$$

[Open Calculator !\[\]\(6a9b39b98eb945faa14c645ec99e4eaa_img.jpg\)](#)

ex
$$1796.272N = (34000 - 88 \cdot 0.5) \cdot 52900mm^2$$



3) Ultimate Load per Area for Aluminium Columns given Allowable Load and Section Area ↗

$$fx \quad P = \left(1.95 \cdot \left(\frac{Q}{A} \right) \right) \cdot A$$

[Open Calculator ↗](#)

$$ex \quad 1234.765N = \left(1.95 \cdot \left(\frac{633.213N}{52900mm^2} \right) \right) \cdot 52900mm^2$$

Axially Loaded Steel Columns Design ↗

4) Allowable Compression Stress given Slenderness Ratio ↗

$$fx \quad F_a = \frac{12 \cdot (\pi^2) \cdot E_s}{23 \cdot (\lambda^2)}$$

[Open Calculator ↗](#)

$$ex \quad 4.325461MPa = \frac{12 \cdot (\pi^2) \cdot 210000MPa}{23 \cdot ((0.5)^2)}$$



5) Allowable Compression Stress when Slenderness Ratio is less than Cc


[Open Calculator](#)

fx $F_a = \frac{1 - \left(\frac{\lambda^2}{2 \cdot C_c^2} \right)}{\left(\frac{5}{3} \right) + \left(3 \cdot \frac{\lambda}{8 \cdot C_c} \right) - \left(\frac{\lambda^3}{8 \cdot (C_c^3)} \right)} \cdot F_y$

ex $16.55172 \text{ MPa} = \frac{1 - \left(\frac{(0.5)^2}{2 \cdot (0.75)^2} \right)}{\left(\frac{5}{3} \right) + \left(3 \cdot \frac{0.5}{8 \cdot 0.75} \right) - \left(\frac{(0.5)^3}{8 \cdot (0.75)^3} \right)} \cdot 40 \text{ MPa}$

6) Slenderness Ratio between Inelastic from Elastic Buckling


[Open Calculator](#)

fx $\lambda = \sqrt{\frac{2 \cdot (\pi^2) \cdot E_s}{F_y}}$

ex $321.9175 = \sqrt{\frac{2 \cdot (\pi^2) \cdot 210000 \text{ MPa}}{40 \text{ MPa}}}$

Cast Iron Columns Design



7) Allowable Load per Area for Cast Iron Columns



fx $Q = (12000 - (60 \cdot \lambda)) \cdot A$

[Open Calculator](#)

ex $633.213 \text{ N} = (12000 - (60 \cdot 0.5)) \cdot 52900 \text{ mm}^2$



8) Critical Slenderness Ratio for Cast Iron Columns ↗

$$fx \quad \lambda = \frac{12000 - \left(\frac{Q}{A} \right)}{60}$$

[Open Calculator ↗](#)

$$ex \quad 0.5 = \frac{12000 - \left(\frac{633.213N}{52900mm^2} \right)}{60}$$

9) Ultimate Load per Area for Cast Iron Columns ↗

$$fx \quad P = (34000 - 88 \cdot (\lambda)) \cdot A$$

[Open Calculator ↗](#)

$$ex \quad 1796.272N = (34000 - 88 \cdot (0.5)) \cdot 52900mm^2$$

Composite Columns ↗

10) Design Strength of Axially Loaded Composite Column ↗

$$fx \quad P_n = 0.85 \cdot A_{Gross} \cdot \frac{F_{cr}}{\Phi}$$

[Open Calculator ↗](#)

$$ex \quad 3060N = 0.85 \cdot 51mm^2 \cdot \frac{60MPa}{0.850}$$

11) Design Strength of Concrete for Direct Bearing ↗

$$fx \quad P_n = 1.7 \cdot \phi_c \cdot A_b \cdot f'_c$$

[Open Calculator ↗](#)

$$ex \quad 2769.3N = 1.7 \cdot 0.6 \cdot 10mm^2 \cdot 271.5MPa$$



12) Gross Area of Steel Core given Design Strength of Axially Loaded Composite Column

fx $A_{\text{Gross}} = P_n \cdot \frac{\Phi}{0.85 \cdot F_{\text{cr}}}$

[Open Calculator !\[\]\(d3fb9f94af8b26d1c844efa9a98805b0_img.jpg\)](#)

ex $50.00017 \text{mm}^2 = 3000.01 \text{N} \cdot \frac{0.850}{0.85 \cdot 60 \text{MPa}}$

13) Loaded Area given Design Strength of Concrete for Direct Bearing

fx $A_b = \frac{P_n}{1.7 \cdot \phi_c \cdot f'_c}$

[Open Calculator !\[\]\(e1d6102fe77919492c04879c8450f1f5_img.jpg\)](#)

ex $10.8331 \text{mm}^2 = \frac{3000.01 \text{N}}{1.7 \cdot 0.6 \cdot 271.5 \text{MPa}}$

Reinforced Concrete Columns

Equivalent Column Concept

14) Curvature of Column Based on Column Mode of Failure

fx $\Phi_m = e_o \cdot \frac{\pi^2}{L^2}$

[Open Calculator !\[\]\(aab88c0d099e5d18d6533a97b13ec28d_img.jpg\)](#)

ex $0.24016 = 219 \text{mm} \cdot \frac{\pi^2}{(3000 \text{mm})^2}$



15) Lateral Deflection of Equivalent Pin Ended Column at distance x 

$$fx \quad e = e_o \cdot \sin\left(\frac{\pi \cdot x}{L}\right)$$

Open Calculator 

$$ex \quad 189.6596mm = 219mm \cdot \sin\left(\frac{\pi \cdot 2000mm}{3000mm}\right)$$

16) Length of Equivalent Pin Ended Column given Max Deflection at Mid Height 

$$fx \quad L = \sqrt{\frac{e_o \cdot \pi^2}{\Phi_m}}$$

Open Calculator 

$$ex \quad 3001.002mm = \sqrt{\frac{219mm \cdot \pi^2}{0.24}}$$

17) Maximum Deflection at Mid Height given Lateral Deflection of Pin Ended Column 

$$fx \quad e_o = \frac{e}{\sin\left(\frac{\pi \cdot x}{L}\right)}$$

Open Calculator 

$$ex \quad 219.3931mm = \frac{190mm}{\sin\left(\frac{\pi \cdot 2000mm}{3000mm}\right)}$$



18) Maximum Deflection at Mid-Height of Equivalent Pin-Ended Column 

$$fx \quad e_o = \Phi_m \cdot \frac{(L)^2}{\pi^2}$$

Open Calculator 

$$ex \quad 218.8538mm = 0.24 \cdot \frac{(3000mm)^2}{\pi^2}$$

Minimum Eccentricity in Design of RCC Columns **19) Axial Load carrying Capacity of Column** 

$$fx \quad P_u = (0.4 \cdot f_{ck} \cdot A_c) + (0.67 \cdot f_y \cdot A_s)$$

Open Calculator **ex**

$$449.75kN = (0.4 \cdot 20MPa \cdot 52450mm^2) + (0.67 \cdot 450MPa \cdot 100.0mm^2)$$

20) Minimum Eccentricity 

$$fx \quad e_{min} = \left(\frac{L}{500} \right) + \left(\frac{b}{30} \right)$$

Open Calculator 

$$ex \quad 21.00033mm = \left(\frac{3000mm}{500} \right) + \left(\frac{450.01mm}{30} \right)$$



21) Unsupported Length of Column given Minimum Eccentricity 

fx
$$L = \left(e_{\min} - \left(\frac{b}{30} \right) \right) \cdot 500$$

Open Calculator 

ex
$$2999.833\text{mm} = \left(21\text{mm} - \left(\frac{450.01\text{mm}}{30} \right) \right) \cdot 500$$



Variables Used

- A Section Area of Column (Square Millimeter)
- A_b Loaded Area (Square Millimeter)
- A_c Area of Concrete (Square Millimeter)
- A_{Gross} Gross Area of Steel Core (Square Millimeter)
- A_s Area of Steel required (Square Millimeter)
- b Least Lateral Dimension (Millimeter)
- C_c Value of C_c
- e Lateral Deflection (Millimeter)
- e_{min} Minimum Eccentricity (Millimeter)
- e_o Maximum Deflection at Mid Height (Millimeter)
- E_s Modulus of Elasticity of Steel (Megapascal)
- F_a Allowable Compression Stress (Megapascal)
- f_c Maximum Compressive Stress of Concrete (Megapascal)
- f_{ck} Characteristic Compressive Strength (Megapascal)
- F_{cr} Critical Compressive Stress (Megapascal)
- f_y Characteristic Strength of Steel Reinforcement (Megapascal)
- F_y Minimum Specified Yield Stress of Steel (Megapascal)
- L Effective Length of Column (Millimeter)
- P Ultimate Load (Newton)
- P_n Nominal Load (Newton)
- P_u Ultimate Axial Load Carrying Capacity of Column (Kilonewton)



- **Q** Allowable Load (*Newton*)
- **x** Distance from One End of Pin Ended Column (*Millimeter*)
- **λ** Slenderness Ratio
- **Φ** Resistance Factor
- **Φ_c** Strength Reduction Factor
- **Φ_m** Curvature of Column



Constants, Functions, Measurements used

- **Constant:** **pi**, 3.14159265358979323846264338327950288
Archimedes' constant
- **Function:** **sin**, sin(Angle)
Trigonometric sine function
- **Function:** **sqrt**, sqrt(Number)
Square root function
- **Measurement:** **Length** in Millimeter (mm)
Length Unit Conversion ↗
- **Measurement:** **Area** in Square Millimeter (mm²)
Area Unit Conversion ↗
- **Measurement:** **Pressure** in Megapascal (MPa)
Pressure Unit Conversion ↗
- **Measurement:** **Force** in Newton (N), Kilonewton (kN)
Force Unit Conversion ↗
- **Measurement:** **Stress** in Megapascal (MPa)
Stress Unit Conversion ↗



Check other formula lists

- Allowable Design for Column Formulas 
- Column Base Plate Design Formulas 
- Columns of Special Materials Formulas 
- Eccentric Loads on Columns Formulas 
- Elastic Flexural Buckling of Columns Formulas 
- Short Axially Loaded Columns with Helical Ties Formulas 
- Ultimate Strength Design of Concrete Columns Formulas 

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