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Ultimate Strength Design of Concrete Columns Formulas

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List of 22 Ultimate Strength Design of Concrete Columns Formulas

Ultimate Strength Design of Concrete Columns

1) 28-day Concrete Compressive Strength given Column Ultimate Strength

$$f'_c = \frac{P_0 - f_y \cdot A_{st}}{0.85 \cdot (A_g - A_{st})}$$

[Open Calculator !\[\]\(a870788d6ed9b8fd294b7654a8c8526b_img.jpg\)](#)

$$\text{ex } 55\text{MPa} = \frac{2965.5\text{MPa} - 250.0\text{MPa} \cdot 7\text{mm}^2}{0.85 \cdot (33\text{mm}^2 - 7\text{mm}^2)}$$

2) Axial Load Capacity of Short Rectangular Members

$$P_u = \Phi \cdot ((.85 \cdot f'_c \cdot b \cdot a) + (A'_s \cdot f_y) - (A_s \cdot f_s))$$

[Open Calculator !\[\]\(c50c8b7b2cc2cf9ff925edec0ee94c0d_img.jpg\)](#)

$$\text{ex } 680.0021\text{N} = 0.850 \cdot ((.85 \cdot 55.0\text{MPa} \cdot 5\text{mm} \cdot 10.5\text{mm}) + (20.0\text{mm}^2 \cdot 250.0\text{MPa}) - (15\text{mm}^2 \cdot 280\text{MPa}))$$

3) Balanced Moment given Load and Eccentricity

$$M_b = e \cdot P_b$$

[Open Calculator !\[\]\(f60b7a900783ac3fd531bfd9c111be6d_img.jpg\)](#)

$$\text{ex } 3.5\text{N}^*\text{m} = 35\text{mm} \cdot 100\text{N}$$

4) Column Ultimate Strength with Zero Eccentricity of Load

$$P_0 = 0.85 \cdot f'_c \cdot (A_g - A_{st}) + f_y \cdot A_{st}$$

[Open Calculator !\[\]\(83bbbd261710c59db0214aa27b2edc0d_img.jpg\)](#)

$$\text{ex } 2965.5\text{MPa} = 0.85 \cdot 55.0\text{MPa} \cdot (33\text{mm}^2 - 7\text{mm}^2) + 250.0\text{MPa} \cdot 7\text{mm}^2$$

5) Compressive Reinforcement Area given Axial-Load Capacity of Short Rectangular Members

$$A'_s = \frac{\left(\frac{P_u}{\Phi}\right) - (.85 \cdot f'_c \cdot b \cdot a) + (A_s \cdot f_s)}{f_y}$$

[Open Calculator !\[\]\(f507db636256ac11a5525ef93ec6b8d7_img.jpg\)](#)

$$\text{ex } 16.79999\text{mm}^2 = \frac{\left(\frac{680\text{N}}{0.850}\right) - (.85 \cdot 55.0\text{MPa} \cdot 5\text{mm} \cdot 10.5\text{mm}) + (15\text{mm}^2 \cdot 280\text{MPa})}{250.0\text{MPa}}$$



6) Tensile Stress in Steel for Axial-Load Capacity of Short Rectangular Members

$$f_s = \frac{(.85 \cdot f'_c \cdot b \cdot a) + (A'_s \cdot f_y) - \left(\frac{P_u}{\Phi}\right)}{A_s}$$

[Open Calculator](#)

$$\text{ex } 443.625\text{MPa} = \frac{(.85 \cdot 55.0\text{MPa} \cdot 5\text{mm} \cdot 10.5\text{mm}) + (20.0\text{mm}^2 \cdot 250.0\text{MPa}) - \left(\frac{680\text{N}}{0.850}\right)}{15\text{mm}^2}$$

7) Tension Reinforcement Area for Axial-Load Capacity of Short Rectangular Members

$$A_s = \frac{(0.85 \cdot f'_c \cdot b \cdot a) + (A'_s \cdot f_y) - \left(\frac{P_u}{\Phi}\right)}{f_s}$$

[Open Calculator](#)

$$\text{ex } 23.76562\text{mm}^2 = \frac{(0.85 \cdot 55.0\text{MPa} \cdot 5\text{mm} \cdot 10.5\text{mm}) + (20.0\text{mm}^2 \cdot 250.0\text{MPa}) - \left(\frac{680\text{N}}{0.850}\right)}{280\text{MPa}}$$

8) Ultimate Strength for Symmetrical Reinforcement

fx

[Open Calculator](#)

$$P_u = 0.85 \cdot f'_c \cdot b \cdot d \cdot \Phi \cdot \left((-\text{Rho}) + 1 - \left(\frac{e'}{d}\right) + \sqrt{\left(\left(1 - \left(\frac{e'}{d}\right)\right)^2\right) + 2 \cdot \text{Rho} \cdot \left(m\right)} \right)$$

ex

$$670.0779\text{N} = 0.85 \cdot 55.0\text{MPa} \cdot 5\text{mm} \cdot 20\text{mm} \cdot 0.85 \cdot \left((-0.5) + 1 - \left(\frac{35\text{mm}}{20\text{mm}}\right) + \sqrt{\left(\left(1 - \left(\frac{35\text{mm}}{20\text{mm}}\right)\right)^2\right) + 2 \cdot \text{Rho} \cdot \left(m\right)} \right)$$

9) Yield Strength of Reinforcing Steel using Column Ultimate Strength

$$f_y = \frac{P_0 - 0.85 \cdot f'_c \cdot (A_g - A_{st})}{A_{st}}$$

[Open Calculator](#)

$$\text{ex } 250\text{MPa} = \frac{2965.5\text{MPa} - 0.85 \cdot 55.0\text{MPa} \cdot (33\text{mm}^2 - 7\text{mm}^2)}{7\text{mm}^2}$$

Circular Columns


10) Eccentricity for Balanced Condition for Short, Circular Members

$$e_b = (0.24 - 0.39 \cdot \text{Rho}' \cdot m) \cdot D$$

[Open Calculator](#)

$$\text{ex } 24.9\text{mm} = (0.24 - 0.39 \cdot 0.9 \cdot 0.4) \cdot 250\text{mm}$$



11) Ultimate Strength for Short, Circular Members when Controlled by Tension 

fx

 Open Calculator 

$$P_u = 0.85 \cdot f'_c \cdot (D^2) \cdot \Phi \cdot \left(\sqrt{\left(\left(\left(0.85 \cdot \frac{e}{D} \right) - 0.38 \right)^2 \right) + \left(\text{Rho}' \cdot m \cdot \frac{D_b}{2.5 \cdot D} \right)} - \left(\left(0.85 \right) \right) \right)$$

ex

$$1.3E^6N = 0.85 \cdot 55.0\text{MPa} \cdot (250\text{mm})^2 \cdot 0.850 \cdot \left(\sqrt{\left(\left(\left(0.85 \cdot \frac{35\text{mm}}{250\text{mm}} \right) - 0.38 \right)^2 \right) + \left(0.9 \cdot 0.4 \cdot \frac{12\text{mm}}{2.5 \cdot 250\text{mm}} \right)} - \left(\left(0.85 \right) \right) \right)$$

 12) Ultimate Strength for Short, Circular Members when Governed by Compression 

fx


 Open Calculator 

$$P_u = \Phi \cdot \left(\left(A_{st} \cdot \frac{f_y}{\left(3 \cdot \frac{e}{D_b} \right) + 1} \right) + \left(A_g \cdot \frac{f'_c}{9.6 \cdot \frac{D_e}{(0.8 \cdot D + 0.67 \cdot D_b)^2} + 1.18} \right) \right)$$

ex

$$0.00018N = 0.850 \cdot \left(\left(7\text{mm}^2 \cdot \frac{250.0\text{MPa}}{\left(3 \cdot \frac{35\text{mm}}{12\text{mm}} \right) + 1} \right) + \left(33\text{mm}^2 \cdot \frac{55.0\text{MPa}}{9.6 \cdot \frac{0.25\text{m}}{(0.8 \cdot 250\text{mm} + 0.67 \cdot 12\text{mm})^2} + 1.18} \right) \right)$$

 Column Strength when Compression Governs 

 13) Ultimate Strength for No Compression Reinforcement 

fx


 Open Calculator 

$$P_u = 0.85 \cdot f'_c \cdot b \cdot d \cdot \Phi \cdot \left((-\text{Rho} \cdot m) + 1 - \left(\frac{e'}{d} \right) + \sqrt{\left(\left(1 - \left(\frac{e'}{d} \right) \right)^2 \right) + 2 \cdot (\text{Rho} \cdot m)} \right)$$

ex

$$689.8837N = 0.85 \cdot 55.0\text{MPa} \cdot 5\text{mm} \cdot 20\text{mm} \cdot 0.85 \cdot \left((-0.5 \cdot 0.4) + 1 - \left(\frac{35\text{mm}}{20\text{mm}} \right) + \sqrt{\left(\left(1 - \left(\frac{35\text{mm}}{20\text{mm}} \right) \right)^2 \right) + 2 \cdot (0.85 \cdot 0.4)} \right)$$




14) Ultimate Strength for Symmetrical Reinforcement in Single Layers 

Open Calculator 

$$P_u = \Phi \cdot \left(\left(A'_s \cdot \frac{f_y}{\left(\frac{e}{d}\right) - d' + 0.5} \right) + \left(b \cdot L \cdot \frac{f'_c}{\left(3 \cdot L \cdot \frac{e}{d^2}\right) + 1.18} \right) \right)$$

ex

$$889.1433\text{N} = 0.85 \cdot \left(\left(20.0\text{mm}^2 \cdot \frac{250.0\text{MPa}}{\left(\frac{35\text{mm}}{20\text{mm}}\right) - 10\text{mm} + 0.5} \right) + \left(5\text{mm} \cdot 3000\text{mm} \cdot \frac{55.0\text{MPa}}{\left(3 \cdot 3000\text{mm} \cdot \frac{35\text{mm}}{(20\text{mm})^2}\right) + 1.18} \right) \right)$$

Short Columns 

15) Ultimate Strength for Short, Square Members when Controlled by Tension 

Open Calculator 

$$P_u = 0.85 \cdot b \cdot L \cdot f'_c \cdot \Phi \cdot \left(\left(\sqrt{\left(\left(\frac{e}{L} - 0.5 \right)^2 \right) + \left(0.67 \cdot \left(\frac{D_b}{L} \right) \cdot \text{Rho}' \cdot m \right)} \right) - \left(\frac{e}{L} \right) \right)$$

ex

$$582742.6\text{N} = 0.85 \cdot 5\text{mm} \cdot 3000\text{mm} \cdot 55.0\text{MPa} \cdot 0.850 \cdot \left(\left(\sqrt{\left(\left(\frac{35\text{mm}}{3000\text{mm}} - 0.5 \right)^2 \right) + \left(0.67 \cdot \left(\frac{12\text{mm}}{3000\text{mm}} \right) \cdot \text{Rho}' \cdot m \right)} \right) - \left(\frac{35\text{mm}}{3000\text{mm}} \right) \right)$$

16) Ultimate Strength for Short, Square Members when Governed by Compression 

Open Calculator 

$$P_u = \Phi \cdot \left(\left(A_{st} \cdot \frac{f_y}{\left(3 \cdot \frac{e}{D_b}\right) + 1} \right) + \left(A_g \cdot \frac{f'_c}{\left(12 \cdot L \cdot \frac{e}{(L+0.67 \cdot D_b)^2}\right) + 1.18} \right) \right)$$

ex

$$1321.976\text{N} = 0.850 \cdot \left(\left(7\text{mm}^2 \cdot \frac{250.0\text{MPa}}{\left(3 \cdot \frac{35\text{mm}}{12\text{mm}}\right) + 1} \right) + \left(33\text{mm}^2 \cdot \frac{55.0\text{MPa}}{\left(12 \cdot 3000\text{mm} \cdot \frac{35\text{mm}}{(3000\text{mm} + 0.67 \cdot 12\text{mm})^2}\right) + 1.18} \right) \right)$$



Slender Columns

17) Axial Load Capacity of Slender Columns

$$\text{fx } P_u = \frac{M_c}{e}$$

[Open Calculator !\[\]\(950a62bbddad88d64435fd35607dfc42_img.jpg\)](#)

$$\text{ex } 680\text{N} = \frac{23.8\text{N}\cdot\text{m}}{35\text{mm}}$$

18) Eccentricity of Slender Columns

$$\text{fx } e = \frac{M_c}{P_u}$$

[Open Calculator !\[\]\(73002692dd5e7a64e60946be3158e719_img.jpg\)](#)

$$\text{ex } 35\text{mm} = \frac{23.8\text{N}\cdot\text{m}}{680\text{N}}$$

19) Magnified Moment given Eccentricity of Slender Columns

$$\text{fx } M_c = e \cdot P_u$$

[Open Calculator !\[\]\(104fbf564e2e5a8fbd84f31656d114c7_img.jpg\)](#)

$$\text{ex } 23.8\text{N}\cdot\text{m} = 35\text{mm} \cdot 680\text{N}$$

Wind Pressure

20) Height given Wind Pressure

$$\text{fx } L = \frac{P}{W_{\text{Column}}}$$

[Open Calculator !\[\]\(097cdd6c9c875b64d9b8c9a2409491c4_img.jpg\)](#)

$$\text{ex } 3000\text{mm} = \frac{72\text{Pa}}{24\text{kN}/\text{m}^3}$$

21) Pressure Walls and Pillars subjected to Wind Pressure

$$\text{fx } p = (W_{\text{Column}} \cdot L)$$

[Open Calculator !\[\]\(13163d77073735089069a7603de98433_img.jpg\)](#)

$$\text{ex } 72\text{Pa} = (24\text{kN}/\text{m}^3 \cdot 3000\text{mm})$$

22) Unit Weight of Material given Wind Pressure

$$\text{fx } W_{\text{Column}} = \frac{p}{L}$$

[Open Calculator !\[\]\(987606e59d5984b3118f78a58e78d0fb_img.jpg\)](#)

$$\text{ex } 24\text{kN}/\text{m}^3 = \frac{72\text{Pa}}{3000\text{mm}}$$










Variables Used

- **a** Depth Rectangular Compressive Stress (Millimeter)
- **A_g** Gross Area of Column (Square Millimeter)
- **A_s** Area of Tension Reinforcement (Square Millimeter)
- **A'_s** Area of Compressive Reinforcement (Square Millimeter)
- **A_{st}** Area of Steel Reinforcement (Square Millimeter)
- **b** Width of Compression Face (Millimeter)
- **d** Distance from Compression to Tensile Reinforcement (Millimeter)
- **d'** Distance from Compression to Centroid Reinforcement (Millimeter)
- **D** Overall Diameter of Section (Millimeter)
- **D_b** Bar Diameter (Millimeter)
- **D_e** Diameter at Eccentricity (Meter)
- **e** Eccentricity of Column (Millimeter)
- **e'** Eccentricity by Method of Frame Analysis (Millimeter)
- **e_b** Eccentricity with respect to Plastic Load (Millimeter)
- **f'_c** 28-Day Compressive Strength of Concrete (Megapascal)
- **f_s** Steel Tensile Stress (Megapascal)
- **f_y** Yield Strength of Reinforcing Steel (Megapascal)
- **L** Effective Length of Column (Millimeter)
- **m** Force Ratio of Strengths of Reinforcements
- **M_b** Balanced Moment (Newton Meter)
- **M_c** Magnified Moment (Newton Meter)
- **p** Columns Pressure (Pascal)
- **P₀** Column Ultimate Strength (Megapascal)
- **P_b** Load Balanced Condition (Newton)
- **P_u** Axial Load Capacity (Newton)
- **Phi** Capacity Reduction Factor
- **Rho** Area Ratio of Tensile Reinforcement
- **Rho'** Area Ratio of Gross Area to Steel Area
- **W_{Column}** Unit weight of RCC Column (Kilonewton per Cubic Meter)
- **Φ** Resistance Factor



Constants, Functions, Measurements used

- **Function:** **sqrt**, $\text{sqrt}(\text{Number})$
Square root function
- **Measurement:** **Length** in Millimeter (mm), Meter (m)
Length Unit Conversion 
- **Measurement:** **Area** in Square Millimeter (mm^2)
Area Unit Conversion 
- **Measurement:** **Pressure** in Pascal (Pa)
Pressure Unit Conversion 
- **Measurement:** **Force** in Newton (N)
Force Unit Conversion 
- **Measurement:** **Moment of Force** in Newton Meter ($\text{N}\cdot\text{m}$)
Moment of Force Unit Conversion 
- **Measurement:** **Specific Weight** in Kilonewton per Cubic Meter (kN/m^3)
Specific Weight Unit Conversion 
- **Measurement:** **Stress** in Megapascal (MPa)
Stress Unit Conversion 



Check other formula lists

- [Allowable Design for Column Formulas](#) 
- [Column Base Plate Design Formulas](#) 
- [Columns of Special Materials Formulas](#) 
- [Eccentric Loads on Columns Formulas](#) 
- [Elastic Flexural Buckling of Columns Formulas](#) 
- [Short Axially Loaded Columns with Helical Ties Formulas](#) 
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